DRAINAGE STATEMENT

For

NAKT Real Estate Holdings, LLC

Proposed Medical Building

Block 473, Lot 1 49 South Avenue West (CR-610) & Lincoln Avenue West Township of Cranford, Union County, New Jersey

Prepared by:



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James E. Henry, PE, PP NJ Professional Engineer License #49266

February 2022 Last Revised September 2022 DEC # 4087-99-001

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- Stormwater Collection System Calculations
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I. Drainage Summary

This Drainage Statement has been prepared to define and analyze the stormwater drainage conditions that would occur as a result of the redevelopment of Block 473, Lot 1 as shown on the Township of Cranford Tax Map Sheet No. 103, located in the Township of Cranford, Union County, New Jersey. The subject site consists of 0.41 acres (17,692 SF) and is located on South Avenue West (CR-610) and Lincoln Avenue West.

Under existing conditions, the site is an abandoned fueling station. The proposed project consists of the construction of a +/- 5,802 SF Medical Building. The first floor is to be +/- 2,836 SF and the second floor +/- 2,966 SF. Additional site improvements will include proposed parking, lighting, landscaping stormwater collection system, grading, driveways, and other associated site amenities. The new development proposes to reduce the impervious coverage on site by approximately 14.3% (2,522SF) and proposes a limit of disturbance of 21,044 SF (0.483 acres).

The project consists of less than one (1) acre of disturbance and will not increase the impervious coverage onsite by ¹/₄ acre or more. Therefore, the proposed project does not meet the definition of a 'major development' and is not subject to the NJDEP Stormwater Management Rules (NJAC 7:8). Furthermore, as the proposed development does not increase the existing impervious surfaces on site it is not subject to the New Jersey Standards for Soil Erosion and Sediment Control runoff rate reduction requirements.

The stormwater drainage for the proposed site has been designed to maintain existing runoff patterns. The stormwater runoff from the proposed development is tributary to the existing stormwater collection system located along South Avenue West (CR-610) and Lincoln Avenue West. The proposed roof leader system of the building routes stormwater runoff to the proposed conveyance system located on the southwestern portion of the subject property, which ultimately discharges into the stormwater conveyance system located on Lincoln Avenue West.

II. <u>Runoff Rate Reduction Performance</u>

As noted above, a majority of the stormwater runoff from the proposed development discharges into the existing stormwater conveyance system located within Lincoln Avenue West. Due to the reduction in impervious coverage associated with the proposed development, the total runoff discharging to this existing stormwater conveyance system decreases for the two (2), ten (10) and one-hundred (100) year design storms modeled as 24-hour SCS Type III design storms as compared to existing conditions. Please refer to the appendix of this report for the hydrography summary reports indicating the same.

Design	Pre-Dev	elopment	Post-Dev	velopment	Reduction in			
Storm	Cond	litions	Flow					
2 Year	0.886	CFS	0.814	CFS	0.072	CFS		
10 Year	1.397	CFS	1.314	CFS	0.083	CFS		
100 Year	2.415	CFS	2.314	CFS	0.101	CFS		

Pre-Development and Post Development Peak Runoff Results Summary
for South Avenue West and Lincoln Ave West

III. Conclusion

The proposed project has been designed to ensure safe and efficient control of the stormwater runoff in a manner that will not adversely impact the existing drainage patterns and systems, adjacent roadways, or adjacent parcels. Furthermore, the project decreases the impervious coverage on-site and thereby does not increase the peak stormwater runoff rates from the parcel. We anticipate that the proposed development will not significantly impact the existing drainage infrastructure located within South Avenue West (CR-610), Lincoln Avenue West, or the surrounding properties.

APPENDIX

NRCS WEB SOIL SURVEY



USDA

Conservation Service



Hydrologic Soil Group

Hydrologic Soil Group— Summary by Map Unit — Union County, New Jersey (NJ039)								
Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI				
HatB	Haledon-Urban land- Hasbrouck complex, 0 to 8 percent slopes	С	0.1	22.5%				
UR	Urban land		0.4	77.5%				
Totals for Area of Inter	est	0.5	100.0%					

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified Tie-break Rule: Higher

RUNOFF CURVE NUMBER (CN) CALCULATIONS – EXISTING



EXISTING DRAINAGE AREA SUMMARY AND AVERAGE CURVE NUMBER(CN) CALCULATIONS

Project: Job #: Location:	Proposed 4087-99-0 Township	Cannabis E 001 of Cranford)ispensary , NJ			Computed Checked E Date:	By: 3y:	PR MD/RTO 9/16/2022											
Drainage Area	Impervious Area (acre)	Impervious Area (sf)	Curve Number (CN) Used	HSG C - Open Space Area (acre)	HSG C - Open Space Area (sf)	Curve Number (CN) Used	HSG C - Wooded Area (acre)	HSG C - Wooded Area (sf)	Curve Number (CN) Used	HSG D - Open Space Area (acre)	HSG D - Open Space Area (sf)	Curve Number (CN) Used	HSG D - Wooded Area (acre)	HSG D - Wooded Area (sf)	Curve Number (CN) Used	Avg. Perv. Curve Number	Total PerviousAr ea (acres)	Total Area (acres)	TC (Min.)
EX. DA LINCOLN AVE WEST	0.35	15,247	98	0.03	1,173	74	0.00	-	70	0.03	1,420	80	0.00		77	77	0.06	0.41	10
Total	0.35	15247.00		0.03	1173.00		0.00	0.00		0.03	1420.00		0.00	0.00			0.06	0.41	
Per County Soil Surve	ey -	HatB	HSG	С	Soil	Haledon-Urb	an land-Hasb	rouck comple	X	1									
Per County Soil Surve	ey -	UR	HSG	D	Soil	Urban land]									
r	Dunef			Dura di	0														

	Runoff Curve Number (CN)	Runoff Curve Number (CN)
Description	(HSG C)	(HSG D)
Impervious Surface	98	98
Open Space (lawn) (good)	74	80
Woods (good)	70	77

RUNOFF CURVE NUMBER (CN) CALCULATIONS – PROPOSED



PROPOSED DRAINAGE AREA SUMMARY AND AVERAGE CURVE NUMBER(CN) CALCULATIONS

Project: Job #: Location:	Proposed 4087-99-0 Township	Cannabis D 01 of Cranford,	ispensary NJ			Computed Checked B Date:	By: 3y:	PR MD/RTO 9/16/2022											
Drainage Area	Impervious Area (acre)	Impervious Area (sf)	Curve Number (CN) Used	HSG C - Open Space Area (acre)	HSG C - Open Space Area (sf)	Curve Number (CN) Used	HSG C - Wooded Area (acre)	HSG C - Wooded Area (sf)	Curve Number (CN) Used	HSG D - Open Space Area (acre)	HSG D - Open Space Area (sf)	Curve Number (CN) Used	HSG D - Wooded Area (acre)	HSG D - Wooded Area (sf)	Curve Number (CN) Used	Avg. Perv. Curve Number	Total PerviousAr ea (acres)	Total Area (acres)	TC (Min.)
PROP. DA LINCOLN AVE WEST	0.29	12,795	98	0.03	1,196	74	0.00	-	70	0.09	3,722	80	0.00	-	77	79	0.11	0.41	10
Total	0.29	12795.00		0.03	1196.00		0.00	0.00		0.09	3722.00		0.00	0.00			0.11	0.41	
Per County Soil Survey	-	HatB	HSG	С	Soil	Haledon-Urb	an land-Hasb	rouck comple	x										
Per County Soil Survey		UR	HSG	D	Soil	Urban land													
							_			-									

	Runoff Curve Number (CN)	Runoff Curve Number (CN)				
Description	(HSG C)	(HSG D)				
Impervious Surface	98	98				
Open Space (lawn) (good)	74	80				
Woods (good)	70	77				

HYDROGRAPH SUMMARY REPORTS – EXISTING AND PROPOSED CONDITIONS 2 YR. & 10 YR.

Thursday, Sep 15, 2022

2 - Year	
Summary Report	. 2
Hydrograph Reports	. 3
Hydrograph No. 1, SCS Runoff, Ex. DA Lincoln Ave West (Per.)	. 3
Hydrograph No. 2, SCS Runoff, Ex. DA Lincoln Ave West (Imp.)	. 4
Hydrograph No. 3, Combine, Ex. DA Lincoln Ave West (Total)	. 5
Hydrograph No. 5, SCS Runoff, Prop. DA Lincoln Ave West (Per.)	6
Hydrograph No. 6, SCS Runoff, Prop. DA Lincoln Ave West (Imp.)	. 7
Hydrograph No. 7, Combine, Prop. DA Lincoln Ave West (Total)	. 8

10 - Year

Summary Report	9
Hydrograph Reports	10
Hydrograph No. 1, SCS Runoff, Ex. DA Lincoln Ave West (Per.)	10
Hydrograph No. 2, SCS Runoff, Ex. DA Lincoln Ave West (Imp.)	11
Hydrograph No. 3, Combine, Ex. DA Lincoln Ave West (Total)	12
Hydrograph No. 5, SCS Runoff, Prop. DA Lincoln Ave West (Per.)	13
Hydrograph No. 6, SCS Runoff, Prop. DA Lincoln Ave West (Imp.)	14
Hydrograph No. 7, Combine, Prop. DA Lincoln Ave West (Total)	15

Watershed Model Schematic





1



Legend

<u>Hyd.</u>	<u>Origin</u>	Description
1	SCS Runoff	Ex. DA Lincoln Ave West (Per.)
2	SCS Runoff	Ex. DA Lincoln Ave West (Imp.)
3	Combine	Ex. DA Lincoln Ave West (Total)
5	SCS Runoff	Prop. DA Lincoln Ave West (Per.)
6	SCS Runoff	Prop. DA Lincoln Ave West (Imp.)
7	Combine	Prop. DA Lincoln Ave West (Total)

Project: 2, 10, 100 yr Hydrograph.gpw

Ŧ	drogra	ph S	mmu	lary F	Report				Hydraftow Hydrographs by Intelisolve v9.1	Hydrograph Report
PAN No	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph description	Hyd. No. 1 Ex. DA Lincoln Ave West (Per.)
	SCS Runoff	0.067	5	730	276				Ex. DA Lincoln Ave West (Per.)	Hydrograph type = SCS Runoff Storm frequency = 2 vrs
0 0	SCS Runoff Combine	0.819 0.886	ഹഗ	730	3,760 4,036	1.2			Ex. DA Lincoln Ave West (Imp.) Ex. DA Lincoln Ave West (Total)	Time interval = 5 min
ſ	SCS Plant	0 125	v	062	25.4				Pron DA Lincoln Ava Maet (Par)	Drainage area = 0.0% Basin Slope = 0.0%
, o	SCS Runoff	0.679	n n	730	3,115	-			Prop. DA Lincoln Ave West (Imp.)	Tc method = USER Total precip. = 3.39 in
7	Combine	0.814	5	730	3,670	5, 6			Prop. DA Lincoln Ave West (Total)	Storm duration = NOAA Atlas
										Q (cfs)
										0.10
										60.0
										80.0
										0.07
										0.06
										0.05
										80
										t
										0.03
										0.02
										0.01
										0 2 4 0 8
										Hyd No. 1
5,	10, 100 yr Hyc	Irograph.	gpw		Return P	eriod: 2 Yeá	ar	Thursday, {	Sep 15, 2022	

Ex. DA Lincoln Ave	e West (Per.)			
Hydrograph type	= SCS Runoff	Peak discharge	= 0.067 cfs	
Storm frequency	= 2 yrs	Time to peak	= 12.17 hrs	
Time interval	= 5 min	Hyd. volume	= 276 cuft	
Drainage area	= 0.060 ac	Curve number	= 77	
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft	
Tc method	= USER	Time of conc. (Tc)	= 10.00 min	
Total precip.	= 3.39 in	Distribution	= Custom	
Storm duration	= NOAA Atlas 14 Type-D.cds	Shape factor	= 484	



2

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Hydraftow Hydrographs by Intelsolve v9.1 Thursday. Sep 16. 2022 Hydr. No. 2 E.X. DA Lincoln Ave West (Imp.) Hydrograph type = SCS Runoff Storm frequency = 2 yrs Time interval = 5 min Drainage area = 0.350 ac Basin Slope = 0.0% Hydraulic length = 0.44 Basin Slope = 0.0% Hydraulic length = 0.6 Hydraulic length = 0.6 Basin Slope = 0.0% Time of conc. (Tc) = 10.00 min Distribution = NOAAMas 14 Type-D.cds Storm duration = NOAAMas 14 Type-D.cds



Hydrograph Report

4

Hydraflow Hydragraphs by Intelsolve v9.1 Thursday, Sep 15, 2022 **Hydr. No. 3** E.: DA Lincoln Ave West (Total) Hydrograph type = Combine Storm frequency = 2 yrs Time to peak = 12.17 hrs Hydr. volume = 4,036 ctfs Time interval = 5 min Inflow hyds. = 1, 2 Contrib. drain. area = 0.410 ac



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Hydraflow Hydrographs by I	Intelisoive v9.1		Thursday, Sep 15, 2022
Hyd. No. 5			
Prop. DA Lincoln A	ve West (Per.)		
Hydrograph type	= SCS Runoff	Peak discharge	= 0.135 cfs
Storm frequency	= 2 yrs	Time to peak	= 12.17 hrs
Time interval	= 5 min	Hyd. volume	= 554 cuft
Drainage area	= 0.110 ac	Curve number	= 79
Basin Slope	= 0.0%	Hydraulic length	= 0 ft
Tc method	= USER	Time of conc. (Tc)	= 10.00 min
Total precip.	= 3.39 in	Distribution	= Custom
Storm duration	= NOAA Atlas 14 Type-D.cds	Shape factor	= 484



Hydrograph Report

9

Hydraflow Hydrographs by Intelisolve v9 1

Hyd. No. 6			
Prop. DA Lincoln A	ve West (Imp.)		
Hydrograph type	= SCS Runoff	Peak discharge	= 0.679 cfs
Storm frequency	= 2 yrs	Time to peak	= 12.17 hrs
Time interval	= 5 min	Hyd. volume	= 3,115 cuft
Drainage area	= 0.290 ac	Curve number	= 98
Basin Slope	= 0.0%	Hydraulic length	= 0 ft
Tc method	= USER	Time of conc. (Tc)	= 10.00 min
Total precip.	= 3.39 in	Distribution	= Custom
Storm duration	= NOAA Atlas 14 Type-D cds	Shape factor	= 484



Thursday, Sep 15, 2022

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Hydraflow Hydrographs by Intelisolve v9.	Hydrograph description	Ex. DA Lincoln Ave West (Per.)	Ex. DA Lincoln Ave West (Imp.)	Ex. DA Lincoln Ave West (Total)	Prop. DA Lincoln Ave West (Per.)	Prop. DA Lincoln Ave West (Imp.)	Prop. DA Lincoln Ave West (Total)	šep 15, 2022
	Total strge used (cuft)							Thursday, S
	Maximum elevation (ft)	İ			1	1	1	ear
_	Inflow hyd(s)	I	ļ	1, 2		ļ	5, 6	eriod: 10 Ye
Inoda	Hyd. volume (cuft)	565	5,876	6,440	1,104	4,868	5,972	Return P
ary n	Time to peak (min)	730	730	730	730	730	730	
	Time interval (min)	5	ъ	5	5	S	ъ	MdE
	Peak flow (cfs)	0.139	1.258	1.397	0.272	1.042	1.314	rograph.(
urograf	Hydrograph type (origin)	SCS Runoff	SCS Runoff	Combine	SCS Runoff	SCS Runoff	Combine	0, 100 yr Hydi
Ň	PAN No	-	2	с	£	9	4	2, 1(



Hydrograph Report Hydraflow Hydrographs by Intelisolve v9.1

Thursday, Sep 15, 2022

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Hyd. No. 7

Prop. DA Lincoln Ave West (Total)

Combine	2 yrs	5 min	5, 6
graph type =	frequency =	nterval =	hyds. =
Hydrog	Storm	Time in	Inflow

Peak discharge = 0.814 cfs Time to peak = 12.17 hrs Hyd. volume = 3,670 cuft Contrib. drain. area = 0.400 ac

(s)					
Q (c		0/.0 -			2.0 -
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Prop					

1.00 Q (cfs)

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0.50 -

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0.30 -

0.20 -

0.10 -

26 Time (hrs) 00.00

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Hyd No. 6 16

Hyd No. 7

6

Hydraflow Hydrographs by	ntelisolve v9.1		Thursday, Sep 15, 2022
Hyd. No. 1			
Ex. DA Lincoln Ave	e West (Per.)		
Hydrograph type	= SCS Runoff	Peak discharge	= 0.139 cfs
Storm frequency	= 10 yrs	Time to peak	= 12.17 hrs
Time interval	= 5 min	Hyd. volume	= 565 cuft
Drainage area	= 0.060 ac	Curve number	= 77
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= USER	Time of conc. (Tc)	= 10.00 min
Total precip.	= 5.17 in	Distribution	= Custom
Storm duration	= NOAA Atlas 14 Type-D.cds	Shape factor	= 484



Hydrograph Report

9

Hydraflow Hydrographs by Intelisolve v9.1

Hyd. No. 2

Ex. DA Lincoln Av∈	e West (Imp.)		
Hydrograph type Storm frequency	= SCS Runoff = 10 yrs	Peak discharge Time to peak	= 1 258 cfs = 12 17 hrs
Time interval	= 5 min	Hyd. volume	= 5,876 cuft
Drainage area	= 0.350 ac	Curve number	= 98
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= USER	Time of conc. (Tc)	= 10.00 min
Total precip.	= 5.17 in	Distribution	= Custom
Storm duration	= NOAA Atlas 14 Type-D.cds	Shape factor	= 484



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Hydraflow Hydrographs by I	ntelisolve v9.1		Thursday, Sep 15, 2022
Hyd. No. 3			
Ex. DA Lincoln Ave	e West (Total)		
Hydrograph type	= Combine	Peak discharge	= 1.397 cfs
Storm frequency	= 10 yrs	Time to peak	= 12.17 hrs
Time interval	= 5 min	Hyd. volume	= 6,440 cuft
Inflow hyds.	= 1, 2	Contrib. drain. area	a = 0.410 ac



12

Hydraflow Hydrographs by Intelisolve v9.1

Hyd. No. 5

on DA Lincoln

⊃rop. DA Lincoln ⊭	we West (Per.)		
Hydrograph type	= SCS Runoff	Peak discharge	= 0.272 cfs
Storm frequency	= 10 yrs	Time to peak	= 12.17 hrs
Time interval	= 5 min	Hyd. volume	= 1,104 cuft
Drainage area	= 0.110 ac	Curve number	= 79
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= USER	Time of conc. (Tc)	= 10.00 min
Total precip.	= 5.17 in	Distribution	= Custom
Storm duration	= NOAA Atlas 14 Type-D cds	Shape factor	= 484



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Hydraflow Hydrographs by	Intelisolve v9.1		Thursday, Sep 15, 2022
Hyd. No. 6			
Prop. DA Lincoln /	Ave West (Imp.)		
Hydrograph type	= SCS Runoff	Peak discharge	= 1.042 cfs
Storm frequency	= 10 yrs	Time to peak	= 12.17 hrs
Time interval	= 5 min	Hyd. volume	= 4,868 cuft
Drainage area	= 0.290 ac	Curve number	= 98
Basin Slope	= 0.0%	Hydraulic length	= 0 ft
Tc method	= USER	Time of conc. (Tc)	= 10.00 min
Total precip.	= 5.17 in	Distribution	= Custom
Storm duration	= NOAA Atlas 14 Type-D cds	Shape factor	= 484



Hydrograph Report

14

Hydraflow Hydrographs by Intelisolve v9.1 Hyd. No. 7

rop. DA Lincoln Ave west (1	Total)		
Hydrograph type = Combil Storm frequency = 10 yrs Time interval = 5 min Inflow byds = 5 6	ei T T C	Deak discharge Time to peak Hyd. volume	= 1.314 cfs = 12.17 hrs = 5,972 cuft = 0.400 ac
)		2



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HYDROGRAPH SUMMARY REPORTS – EXISTING AND PROPOSED CONDITIONS 100 YR.

tershed Model Schematic 1	

100 - Year	
Summary Report	2
Hydrograph Reports	3
Hydrograph No. 1, SCS Runoff, Ex. DA Lincoln Ave West (Per.)	3
Hydrograph No. 2, SCS Runoff, Ex. DA Lincoln Ave West (Imp.)	4
Hydrograph No. 3, Combine, Ex. DA Lincoln Ave West (Total)	5
Hydrograph No. 5, SCS Runoff, Prop. DA Lincoln Ave West (Per.)	6
Hydrograph No. 6, SCS Runoff, Prop. DA Lincoln Ave West (Imp.)	7
Hydrograph No. 7, Combine, Prop. DA Lincoln Ave West (Total)	8

Watershed Model Schematic



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<u>Legend</u>

<u>Hyd.</u>	<u>Origin</u>	Description
1	SCS Runoff	Ex. DA Lincoln Ave West (Per.)
2	SCS Runoff	Ex. DA Lincoln Ave West (Imp.)
3	Combine	Ex. DA Lincoln Ave West (Total)
5	SCS Runoff	Prop. DA Lincoln Ave West (Per.)
6	SCS Runoff	Prop. DA Lincoln Ave West (Imp.)
7	Combine	Prop. DA Lincoln Ave West (Total)

Project: 2, 10, 100 yr Hydrograph.gpw

Hydrogra	ph Sı	amm Tamma	ary R	Report				2 Hudraffow Hudronranhe hu Intaliendue vo 1	Hydrograph Report	n
Hyd. Hydrograph No. type (origin)	Peak flow (cfs)	Time interval (min)	Time to peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Hydrograph description	Hydrattow Hydrographs by Intelsolve v9.1 Hyd. No. 1 Ex. DA Lincoln Ave West (Per.)	Thursday, Sep 15, 2022
1 SCS Runoff 2 SCS Runoff 3 Combine 5 SCS Runoff 6 SCS Runoff 7 Combine	0.293 2.122 2.415 0.556 1.758 2.314	າ ເກັດ ເມັດ ເມັດ	730 730 730 730 730	1,207 10,065 11,271 2,303 8,339 10,643	с о с о			Ex. DA Lincoln Ave West (Per.) Ex. DA Lincoln Ave West (Inp.) Ex. DA Lincoln Ave West (Total) Prop. DA Lincoln Ave West (Per.) Prop. DA Lincoln Ave West (Total) Prop. DA Lincoln Ave West (Total)	Hydrograph type= SCS RunoffPeak dischargeStorm frequency= 100 yrsTime to peakStorm frequency= 5 minHyd. volumeTime interval= 5 minHyd. volumeDrainage area= 0.060 acHydraulic lengthDrainage area= 0.0%Hydraulic lengthTo method= U.S.USTo method= 8.69 inDistributionStorm duration= NOAAAtlas 14 Type-D.cdsShape factor	= 0.293 cfs = 12.17 hrs = 1,207 cuft = 7 = 0 ff = 0 ff = 10.00 min = 484
									G(5) Ex. DA Lincoln Ave West (Per.) 9(6) 946 030 945 046 946	α (c 0.56 0.46
2, 10, 100 yr Hyc	drograph.c	Md		Return P	Period: 100 Y	Year	Thursday, S	iep 15, 2022	—— Hyd No. 1	

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Hydraftow Hydrographs by Intelsolve v9.1 Thursday, Sep 15, 2022 Hydr. No. 2 E.X. DA Lincoln Ave West (Imp.) Hydrograph type = SCS Runoff Storm frequency = 100 yrs Time interval = 5 min Drainage area = 0.350 ac Basin Slope = 0.0% Basin Slope = 0.0% Hydr. volume = 10,065 cuft Hydr. volume = 98 Hydr. volume = 98 Hydraulic length = 0,0 Time of corrc. (Tc) = 10,00 min Total precip. = 8.69 in Storm duration = NOAAAtlas 14 Type-D.cds Storm duration = NOAAAtlas 14 Type-D.cds



Hydrograph Report

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Hydraftow Hydraftow Val. Thursday, Sep 15. 2022 **Hydr. No. 3** E.v. DA Lincoln Ave West (Total) Hydrograph type = Combine Storm frequency = 100 vrs Time interval = 5 min Inflow hyds. = 1, 2 Contrib. drain. area = 0.410 ac



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Hydraffow Hydrographs by I	ntelisolve v9.1		Thursday, Sep 15, 2022
Hyd. No. 5			
Prop. DA Lincoln A	we West (Per.)		
Hydrograph type	= SCS Runoff	Peak discharge	= 0.556 cfs
Storm frequency	= 100 yrs	Time to peak	= 12.17 hrs
Time interval	= 5 min	Hyd. volume	= 2,303 cuft
Drainage area	= 0.110 ac	Curve number	= 79
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= USER	Time of conc. (Tc)	= 10.00 min
Total precip.	= 8.69 in	Distribution	= Custom
Storm duration	= NOAA Atlas 14 Type-D.cds	Shape factor	= 484



Hydrograph Report

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Hydraflow Hydrographs by Intelisolve v9.1

Hyd. No. 6

DAlin

Prop. DA Lincoln A	we West (Imp.)			
Hydrograph type	= SCS Runoff	Peak discharge	= 1.758 cfs	
Storm frequency	= 100 yrs	Time to peak	= 12.17 hrs	
Time interval	= 5 min	Hyd. volume	= 8,339 cuft	
Drainage area	= 0.290 ac	Curve number	= 98	
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft	
Tc method	= USER	Time of conc. (Tc)	= 10.00 min	
Total precip.	= 8.69 in	Distribution	= Custom	
Storm duration	= NOAA Atlas 14 Type-D cds	Shape factor	= 484	



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Hydraflow Hydrographs by Intelisolve v9.1	Hyd. No. 7

Prop. DA Lincoln Ave West (Total)

Hydrograph type Storm frequency Time interval Inflow hvds.	= Combine = 100 yrs = 5.6

2.314 cfs	12.17 hrs	10,643 cuft	0.400 ac
ш	н	н	П
Peak discharge	Time to peak	Hyd. volume	Contrib drain area



STORMWATER COLLECTION SYSTEM CALCULATIONS



Stormwater Collection System Calculations Project: Proposed Medical Building Computed By: MD

Project: Proposed Medical Building Job #: 4087-99-001 Location: Township of Cranford, Union County, NJ Design Storm: 25 Year Computed By: MD Checked By: RTO Date: 2/25/2022 NOTES:

 Design method used is Rational Method, unless otherwise noted.
 Refer to Weighted Runoff Coefficient table for calculation of incremental areas and C values

PIPE SE	ECTION	SUBCATCHMENT AREA	INCR	REMENTAL	CUMULATIVE	CON	TIME OF CENTRA	TION	Ι	PEAK R	UNOFF	PIP	PING INP	UT	P	IPING DAT	ГА
FROM	ТО	Area (Acres)	"C"	A x C Ac	A x C (acres)	Tc to Inlet (min)	Tc in Pipe (min.)	Final Tc (min)	(In/Hr)	Q to Inlet (CFS)	Q cum. for Pipe (CFS)	Dia. (In)	Length (Ft)	Man. "n"	Slope (ft/ft)	Pipe Capacity (cfs)	Pipe Velocity (fps)
Inlet Area Roof	Inlet Area 1	0.07	0.95	0.07	0.07	10.00	0.52	10.00	6.80	0.48	0.48	6	115.0	0.010	0.0100	0.73	3.72
Inlet Area 1	Inlet Area 2	0.11	0.83	0.09	0.16	10.00	0.53	10.52	6.68	0.60	1.07	15	119.0	0.013	0.0050	4.57	3.73
Inlet Area 2	Existing Inlet	0.07	0.95	0.07	0.23	10.00	0.08	11.05	6.56	0.46	1.51	15	17.0	0.013	0.0050	4.57	3.73

DRAINAGE AREA MAPS

SYMBOL LEGEND							
	CONC. MONUMENT FND		MAIL BOX				
0	I.P. / I.B. FND	(GA)	CABLE TV BOX				
O	TACK / STAKE FND	A	TELEPHONE BOX				
× 10 ^{3.55}	SPOT ELEVATIONS	\square	A/C UNIT				
¥	TRAFFIC SIGNAL POLE	\boxtimes	TRANSFORMER				
J.	UTILITY POLE	E	ELECTRIC METER				
>	GUY WIRE	G	GAS METER				
o ê	UTILITY POLE W/LIGHT	W	WATER METER				
¢	LIGHT POLE		WATER VALVE				
-0	SIGN	Ø	GAS VALVE				
- Ch	FIRE HYDRANT	0	CLEAN OUT				
D. W.P.	DETECTABLE WARNING PAD	×c	GAS				
D. C.	DEPRESSED CURB	\times #	WATER				
L.S.A.	LANDSCAPED AREA	$\varkappa \varepsilon$	ELECTRIC				
	WELL	$\times r$	TELEPHONE				
\bigcirc	MANHOLE	XC	CABLE TV				
	"A "—INLET		TREE				
	"B"-INLET	$\overline{\mathbb{C}}$	SHRUB				
	"E"-INLET	۲	BOLLARD				
	YARD INLET	Ð	MONITORING WELL				
\square	FLARED END SECTION		WETLAND FLAG				

GENERAL NOTES:

- 1. THIS SURVEY IS PREPARED IN ACCORDANCE WITH DOCUMENTS SUPPLIED BY THE CLIENT AND THOSE OBTAINED THROUGH SUPPLEMENTAL RESEARCH BY DPK. THE DOCUMENTS UTILIZED MAY OR MAY NOT REPRESENT ALL THE TITLE DOCUMENTS RELEVANT TO THE SUBJECT PROPERTY. IT IS STRONGLY SUGGESTED THAT A COMPLETE TITLE SEARCH BE SUPPLIED TO
- THE SURVEYOR FOR REVIEW PRIOR TO THE PLACEMENT OF OR ALTERATION TO IMPROVEMENTS ON THE PROPERTY. 2. THIS SURVEY IS SUBJECT TO ANY EASEMENTS OF RECORD AND ANY OTHER PERTINENT FACTS THAT A COMPLETE TITLE SEARCH MIGHT DISCLOSE.
- 3. THIS SURVEY REPRESENTS FIELD CONDITIONS AS OF NOVEMBER
- 17, 2021.
 THE UTILITIES SHOWN HAVE BEEN LOCATED FROM EVIDENCE OBSERVED ON THE SURFACE ONLY OR HAVE BEEN SHOWN GRAPHICALLY PER SUPPLIED MATERIALS. DPK CONSULTING MAKES NO GUARANTEES THAT THE UTILITIES SHOWN COMPRISE ALL SUCH UTILITIES IN THE AREA, EITHER IN-SERVICE OR ABANDONED. DPK CONSULTING FURTHER DOES NOT WARRANT THAT THE UNDERGROUND UTILITIES SHOWN ARE IN THE EXACT LOCATION INDICATED. DPK CONSULTING HAS NOT PHYSICALLY LOCATED THE UNDERGROUND UTILITIES. PREMISES ARE COMMONLY KNOWN AS 49 SOUTH AVENUE WEST,
- 5. CRANFORD, NEW JERSEY.
- 6. ALSO KNOWN AS LOT 1 IN BLOCK 473 AS SHOWN ON THE OFFICIAL TAX MAPS OF THE TOWNSHIP OF CRANFORD, UNION
- COUNTY, NEW JERSEY. 7. CORNER MARKERS AS SHOWN HAVE BEEN LOCATED, VERIFIED
- AND/OR SET. 8. THE PROJECT VERTICAL DATUM IS BASED UPON NAVD 88 DERIVED USING LEICA GX1230 GPS RECEIVERS AND NEW JERSEY
- SMARTNET NETWORK. 9. IF THIS DOCUMENT DOES NOT CONTAIN A RAISED SEAL OF THE UNDERSIGNED PROFESSIONAL, IT IS NOT AN AUTHORIZED ORIGINAL DOCUMENT.







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rion)			
UGET UG	ET	× 8.9	EXIST. SPOT ELEVATIONS
UGET	PROP. UNDERGROUND ELEC./TELE. SERVICE (NO. & SIZE OF CONDUITS NOT DEFINED)	× G: 8.90 × TC: 8.90 × FF: 8.90	EXIST. GUITER ELEV. EXIST. TOP OF CURB ELEV. EXIST. FINISH FLOOR ELEV.
	EXIST. SANITARY SEWER LINE	× GF: 8.90	EXIST. GARAGE FLOOR ELEV
SSS	PROP. SANITARY SEWER LINE	G: 000.00 TC: 000.00	PROP. GRADE SPOT ELEV.
	EXIST. STORM DRAIN LINE	G: 000.00 FF: 000.00	PROP. FINISHED FLOOR ELE
	EXIST. MINOR CONTOUR & ELEVATION	TW: 000.00 GL: 000.00	PROP. TOP OF WALL & FINISHE OF WALL (ACTUAL BOTTOM C BE ESTABLISHED BY WALL
X	→ PROP. FINISH GRADE CONTOUR & ELEVATIO → PROP. DIRECTION OF DRAINAGE FLOW ARRO	N TC: 000.00 GH: 000.00 W GL: 000.00	PROP. TOP OF EXTENDED CURB @ HIGH SIDE OF EXTENDED CU GRADE @ LOW SIDE OF EXTEND
	10N) UGET UGET UGET S OR S S S OR S OR S S S S S S S S S	ION) UGET UGET EXIST. UNDERGROUND ELEC./TELE. SERVICE (NO. & SIZE OF CONDUITS NOT DEFINED) UGET PROP. UNDERGROUND ELEC./TELE. SERVICE (NO. & SIZE OF CONDUITS NOT DEFINED) S OR EXIST. SANITARY SEWER LINE NOR S PROP. SANITARY SEWER LINE NOR EXIST. STORM DRAIN LINE PROP. STORM DRAIN LINE EXIST. MINOR CONTOUR & ELEVATION EXIST. MAJOR CONTOUR & ELEVATION EXIST. MAJOR CONTOUR & ELEVATION EXIST. MAJOR CONTOUR & ELEVATION PROP. DIRECTION OF DRAINAGE FLOW ARROW	ION) UGET UGET EXIST. UNDERGROUND ELEC./TELE. SERVICE (NO. & SIZE OF CONDUITS NOT DEFINED) × 8.9 UGET PROP. UNDERGROUND ELEC./TELE. SERVICE (NO. & SIZE OF CONDUITS NOT DEFINED) × 6: 8.90 S OR EXIST. SANITARY SEWER LINE × 6: 8.90 S OR EXIST. SANITARY SEWER LINE × 6: 8.90 PROP. UNDERGROUND ELEC./TELE. SERVICE (NO. & SIZE OF CONDUITS NOT DEFINED) × 6: 8.90 × 6: 8.90 S OR EXIST. SANITARY SEWER LINE × 6: 8.90 S OR EXIST. SANITARY SEWER LINE × 6: 8.90 Y OR EXIST. SANITARY SEWER LINE × 6: 8.90 Y OR EXIST. SANITARY SEWER LINE ITC: 000.00 G: 000.00 EXIST. STORM DRAIN LINE FF: 000.00 PROP. STORM DRAIN LINE FF: 000.00 ITW: 000.00 IXX MJOR CONTOUR & ELEVATION ITW: 000.00 PROP. FINISH GRADE CONTOUR & ELEVATION ITW: 000.00 ITW: 000.00 OR OR PROP. DIRECTION OF DRAINAGE FLOW ARROW ITW: 000.00

UTILITY NOTES

- 1. LOCATION OF ALL EXISTING AND PROPOSED SERVICES ARE APPROXIMATE AND MUST BE CONFIRMED INDEPENDENTLY WITH LOCAL UTILITY COMPANIES PRIOR TO COMMENCEMENT OF ANY CONSTRUCTION OR EXCAVATION. SANITARY SEWER AND ALL OTHER UTILITY SERVICE CONNECTION POINTS SHALL BE CONFIRMED INDEPENDENTLY BY THE CONTRACTOR IN FIELD PRIOR TO THE COMMENCEMENT OF CONSTRUCTION. ALL DISCREPANCIES SHALL BE REPORTED IMMEDIATELY IN WRITING TO THE ENGINEER. CONSTRUCTION SHALL COMMENCE BEGINNING AT THE LOWEST INVERT (POINT OF CONNECTION) AND PROGRESS UP GRADIENT. INTERFACE POINTS (CROSSINGS) WITH EXISTING UNDERGROUND UTILITIES SHALL BE FIELD VERIFIED BY TEST PIT PRIOR TO COMMENCEMENT OF CONSTRUCTION.
- 2. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY UTILITY "ONE-CALL" NUMBER 72 HOURS PRIOR TO ANY EXCAVATION ON THIS SITE. CONTRACTOR SHALL ALSO NOTIFY LOCAL WATER & SEWER DEPARTMENTS TO MARK-OUT THEIR UTILITIES.
- 3. REFER TO ARCHITECTURAL DRAWINGS FOR EXACT BUILDING UTILITY CONNECTION LOCATIONS. WHERE CONFLICTS EXIST WITH THESE SITE PLANS, ENGINEER IS TO BE NOTIFIED PRIOR TO CONSTRUCTION TO RESOLVE SAME. SERVICE SIZES TO BE DETERMINED BY ARCHITECT.
- 4. WATER SERVICE MATERIALS SHALL BE SPECIFIED BY THE LOCAL UTILITY COMPANY. CONTRACTORS PRICE FOR WATER SERVICE SHALL INCLUDE ALL FEES AND APPURTENANCES REQUIRED BY THE UTILITY TO PROVIDE A COMPLETE WORKING SERVICE.
- 5. ALL WATER MAIN SHALL BE CEMENT-LINED, CLASS 52 DUCTILE IRON PIPE, UNLESS OTHERWISE DESIGNATED.
- 6. THE MINIMUM DIAMETER FOR DOMESTIC WATER SERVICES SHALL BE 1 INCH.
- 7. SEWER MAINS SHALL BE SEPARATED FROM WATER MAINS BY A DISTANCE OF AT LEAST 10 FEET HORIZONTALLY. WHERE THIS IS NOT POSSIBLE, THE PIPES SHALL BE IN SEPARATE TRENCHES WITH THE SEWER MAIN AT LEAST 18 INCHES BELOW THE WATER MAIN. ALL SEWER MAINS SHALL BE SDR-35 PVC PIPE UNLESS OTHERWISE DESIGNATED.
- 8. ALL SEWER PIPE INSTALLED WITH LESS THAN 3 FEET OF COVER, GREATER THAN 20 FEET OF COVER OR WITHIN 18 INCHES OF A WATER MAIN SHALL BE CONSTRUCTED OF DUCTILE IRON PIPE. ALL DUCTILE IRON SEWER PIPE SHALL BE CEMENT-LINED, CLASS 52 PIPE, FURNISHED WITH SEWER COAT, OR APPROVED
- 9. WHERE SANITARY SEWER LATERALS ARE GREATER THAN 10' DEEP AT CONNECTION TO THE SEWER MAIN, CONCRETE DEEP LATERAL CONNECTIONS ARE TO BE UTILIZED.
- 10. THE CONTRACTOR IS RESPONSIBLE FOR THE STABILIZATION OF THE EXISTING SEWER MAIN, STRUCTURES AND APPURTENANCES DURING CONNECTION. 11. LOCATION & LAYOUT OF GAS, ELECTRIC & TELECOMMUNICATION UTILITY LINES AND SERVICES SHOWN ON THESE PLANS ARE SCHEMATIC IN NATURE. ACTUAL LOCATION & LAYOUT OF THESE UTILITIES & SERVICES ARE TO BE PER THE APPROPRIATE UTILITY PROVIDER.
- 12. ROOF LEADER COLLECTION PIPING ARE CONCEPTUAL IN NATURE AND ARE NOT FOR CONSTRUCTION. ACTUAL ROOF LEADER COLLECTION PIPING IS TO BE COORDINATED W/ ARCHITECTURAL PLANS FOR EACH INDIVIDUAL BUILDING. ALL ROOF LEADER COLLECTION PIPING SHALL BE SCHEDULE 40 PVC UNLESS OTHERWISE DESIGNATED.
- 13. ALL SEWER AND WATER FACILITIES SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE REGULATORY AUTHORITY'S RULES AND REGULATIONS.
- 14. ALL PROPOSED UTILITIES TO BE INSTALLED UNDERGROUND UNLESS OTHERWISE NOTED.
- 15. MANUFACTURED REINFORCED CONCRETE STORM PIPE TO CONFORM TO ASTM C-76, CLASS III, UNLESS OTHERWISE DESIGNATED. MANUFACTURED REINFORCED CONCRETE ELLIPTICAL STORM PIPE TO CONFORM TO ASTM C-507, CLASS HE-III, UNLESS OTHERWISE DESIGNATED. REINFORCED CONCRETE STORMWATER PIPE TO BE INSTALLED IN ACCORDANCE WITH AMERICAN CONCRETE PIPE ASSOCIATION INSTALLATION GUIDELINES AND MORTAR OR PREFORMED FLEXIBLE JOINT SEALANTS IN ACCORDANCE WITH ASTM C 990 TO BE UTILIZED TO PROVIDE A SILT-TIGHT JOINT. WHERE SPECIFICALLY INDICATED, REINFORCED CONCRETE STORM PIPE JOINTS SUMU DE WITH TOTICUL TO DESTIN C 457 A SHALL BE WATERTIGHT AND CONFORM TO ASTM C-443.
- 16. HDPE DRAINAGE PIPE SHALL HAVE A SMOOTH WALL INTERIOR WITH ANNULAR EXTERIOR CORRUGATIONS AND CONFORM TO ASTM F2306. SOLID PIPE SHALL HAVE GASKETED WATER-TIGHT JOINTS MEETING THE REQUIREMENTS OF ASTM F2306 AND ASTM D3212. PERFORATED PIPE SHALL HAVE GASKETED SILT-TIGHT JOINTS MEETING THE REQUIREMENTS OF ASTM F2306 AND ASTM F477. HDPE PIPE SHALL BE FROM A MANUFACTURER WHO IS AN EASTERN STATES CONSORTIUM (ESC) QUALIFIED MANUFACTURER OF HDPE PIPE AND INSTALLED IN ACCORDANCE WITH PIPE MANUFACTURE RECOMMENDATIONS.
- 17. HP DRAINAGE PIPE SHALL HAVE A SMOOTH WALL INTERIOR WITH ANNULAR EXTERIOR CORRUGATIONS AND CONFORM TO ASTM F2736 (12"-30" PIPE) AND ASTM F2881 (36"-60" PIPE). PIPE SHALL HAVE GASKETED WATER-TIGHT JOINTS MEETING THE REQUIREMENTS OF ASTM D3212 AND ASTM F477. FIELD WATERTIGHTNESS PERFORMANCE VERIFICATION MAY BE ACCOMPLISHED IN ACCORDANCE WITH ASTM F2487. HP PIPE SHALL BE FROM A MANUFACTURER WHO IS AN EASTERN STATES CONSORTIUM (ESC) QUALIFIED MANUFACTURER OF HP STORM PIPE AND INSTALLED IN ACCORDANCE WITH PIPE MANUFACTURER RECOMMENDATIONS.
- 18. PIPE LENGTHS ON THIS PLAN HAVE BEEN MEASURED AS THE DISTANCE BETWEEN THE CENTER POINT OF THE 2 CONNECTED STRUCTURES. ACTUAL PHYSICAL PIPE LENGTH FOR INSTALLATION IS EXPECTED TO BE LESS AND SHOULD BE ACCOUNTED FOR BY THE CONTRACTOR ACCORDINGLY.

EXISTING SANITARY NOTES

- 1. APPROXIMATE LOCATION OF 12" SANITARY SEWER MAIN AND DROP MANHOLE AS PER THE TOWNSHIP OF CRANFORD EXISTING SANITARY SEWERS MAPPING, DATED JANUARY 1977. INVERTS SHALL BE CONFIRMED BY THE CONTRACTOR PRIOR TO
- CONSTRUCTION. 2. THE INVERT OF THE SANITARY SEWER MAIN WAS INTERPOLATED PER THE INFORMATION ON THE TOWNSHIP OF CRANFORD
- EXISTING SANITARY SEWERS MAPPING, DATED JANUARY 1977. CONTRACTOR TO CONFIRM DEPTH OF SANITARY SEWER MAIN INVERT PRIOR TO CONSTRUCTION. IF THE ELEVATION ONSITE VARIES FROM WHAT IS SHOWN ON THIS PLAN, DYNAMIC
- ENGINEERING MUST BE CONTACTED IMMEDIATELY. 3. THE INVERT WAS DETERMINED THROUGH DIFFERENTIAL CALCULATIONS OF THE INFORMATION FROM THE TOWNSHIP OF CRANFORD EXISTING SANITARY SEWERS MAPPING, DATED JANUARY 1977. INVERTS SHALL BE CONFIRMED BY THE CONTRACTOR PRIOR TO CONSTRUCTION.

NOTES

CONNECTION.

- 1. INSTALLATION OF SANITARY MANHOLE IN LINCOLN AVE TO BE COORDINATED WITH CRANFORD DPW AND POLICE PRIOR TO BEGINNING WORK IN R.O.W.
- 2. PRIOR TO COMMENCEMENT OF CONSTRUCTION, CONTRACTOR AND ARCHITECT SHALL CONFIRM THE SIZE, CAPACITY AND CONDITION OF THE EXISTING UTILITIES IN ORDER TO CONFIRM IF THE EXISTING UTILITIES CAN
- BE UTILIZED FOR THIS DEVELOPMENT. 3. A ROAD OPENING PERMIT IS REQUIRED PRIOR TO CONSTRUCTION WITHIN THE R.O.W. INCLUDING INSTALLATION OF THE DRIVEWAY APRON, R.O.W. CURBING, R.O.W. SIDEWALK AND SANITARY SEWER

1 INCH = 20 FT.							
	07719						
UD ENIGINICE FINIC	4.0198 74.3521 ec.com						
LAND DEVELOPMENT CONSULTING • PERMITTING • GEOTECHNICAL • ENVIRONMENTAL • SURVEY • PLANNING & ZON	ING						
Offices conveniently located in: Lake Como, New Jersey T: 732.974.0198 Chester, New Jersey T: 908.879.9229 Newark, New Jersey T: 973.755.7200 Toms River, New Jersey T: 732.678.0000							
Allen, Texas T: 972.534.2100 Austin, Texas T:512.646.2646 Houston, Texas T: 281.789.6400 Delray Beach, Florida T: 561.921.8570 Newtown, Pennsylvania T: 267.685.0276 Philadelphia, Pennsylvania T: 215.253.4888 Bethlehem, Pennsylvania T: 610.598.4400							
PROPOSED DRAINAGE AREA MAP EXHIBIT							
PROJECT: NAKT REAL ESTATE HOLDINGS. LLC JOB NO: 1007 00 001							
PROPOSED MEDICAL BUILDING	022						
BLOCK 473, LOT 1 49 SOUTH AVENUE WEST (CR 610) & LINCOLN AVENUE WEST DJB	=20						
TOWNSHIP OF CRANFORD, UNION COUNTY, NEW JERSEY DESIGNED BY:							
CHECKED BY:							
PROFESSIONAL ENGINEER PROFESSIONAL ENGINEER							
NEW JERSEY LICENSE No. 49266 NEW JERSEY LICENSE No. 41975 FOR STATE SPECIFIC DIRECT PHONE NUMBERS VISIT: Rev. # O							

INLET AREA MAP



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XIST. CABLE LINE	UGET UGET	EXIST. UNDERGROUND ELEC./TELE. SERVICE	× 8.9	EXIST. SPOT ELEVATION
ROP. CABLE LINE		(NO. & SIZE OF CONDUITS NOT DEFINED)	× G: 8.90	EXIST. GUTTER ELEV.
XIST. ELECTRIC LINE	UGET	PROP. UNDERGROUND ELEC./TELE. SERVICE	× TC: 8.90	EXIST. TOP OF CURB E
ROP. ELECTRIC LINE		(NO. & SIZE OF CONDUITS NOT DEFINED)	× FF: 8.90	EXIST. FINISH FLOOR E
XIST. FIBER OPTIC LINE	S S	EXIST. SANITARY SEWER LINE	×GF: 8.90	EXIST. GARAGE FLOOR
ROP. FIBER OPTIC LINE			G: 000.00	PROP. GRADE SPOT EL
XIST. GAS LINE	S S	PROP. SANITARY SEWER LINE	TC: 000.00	
ROP. GAS LINE			G: 000.00	PROP. TOP OF CORB &
XIST. OVERHEAD WIRES		EXIST. STORM DRAIN LINE	FF: 000.00	PROP. FINISHED FLOOR
ROP. OVERHEAD WIRES		PROP. STORM DRAIN LINE		PROP. TOP OF WALL & FI
XIST. TELEPHONE LINE		EXIST. MINOR CONTOUR & ELEVATION	GL: 000.00	OF WALL (ACTUAL BOTTO
ROP TELEPHONE LINE	10	EXIST. MAJOR CONTOUR & ELEVATION		BE ESTABLISHED BY W
VIST WATER LINE	XX	PROP. FINISH GRADE CONTOUR & ELEVATION	TC: 000.00	PROP. TOP OF EXTENDED
		PROP DIRECTION OF DRAINAGE FLOW ARROW		CRADE @ LOW SIDE OF FY

GRADING NOTES

- 1. SITE GRADING SHALL BE PERFORMED IN ACCORDANCE WITH THESE PLANS AND SPECIFICATIONS AND THE RECOMMENDATIONS SET FORTH IN THE SOILS REPORT REFERENCED IN THIS PLAN SET. THE CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVING AND REPLACING ALL SOFT, YIELDING OR UNSUITABLE MATERIALS AND REPLACING WITH SUITABLE MATERIALS AS SPECIFIED IN THE SOILS REPORT. ALL EXCAVATED OR FILLED AREAS SHALL BE COMPACTED TO 95% OF MODIFIED PROCTOR MAXIMUM DENSITY PER A.S.T.M. TEST D-1557. MOISTURE CONTENT AT TIME OF PLACEMENT SHALL NOT EXCEED 2% ABOVE NOR 3% BELOW OPTIMUM. CONTRACTOR SHALL SUBMIT A COMPACTION REPORT PREPARED BY A QUALIFIED SOILS ENGINEER, REGISTERED WITHIN THE STATE WHERE THE WORK IS PERFORMED, VERIFYING THAT ALL FILLED AREAS AND SUBGRADE AREAS WITHIN THE BUILDING PAD AREA AND AREAS TO BE PAVED HAVE BEEN COMPACTED IN ACCORDANCE WITH THESE PLANS AND SPECS AND THE RECOMMENDATIONS SET FORTH IN THE SOILS REPORT.
- 2. CONTRACTOR IS RESPONSIBLE FOR VERIFICATION OF EXISTING TOPOGRAPHIC INFORMATION AND UTILITY INVERT ELEVATIONS PRIOR TO COMMENCEMENT OF ANY CONSTRUCTION. CONTRACTOR TO ENSURE 0.75% MIN. SLOPE AGAINST ALL ISLAND GUTTERS, CURBS AND 1.0% ON ALL CONCRETE SURFACES, AND 1–1/2% MIN. ON ASPHALT, TO PREVENT PONDING. ANY DISCREPANCIES THAT MAY EFFECT THE PUBLIC SAFETY OR PROJECT COST, MUST BE IDENTIFIED TO THE ENGINEER IN WRITING IMMEDIATELY. PROCEEDING WITH CONSTRUCTION WITH DESIGN DISCREPANCIES IS DONE SO AT THE CONTRACTOR'S OWN RISK.
- 3. PROPOSED TOP OF CURB ELEVATIONS ARE GENERALLY 6" ABOVE EXISTING LOCAL ASPHALT GRADE UNLESS OTHERWISE NOTED. FIELD ADJUST TO CREATE A MIN. OF 0.75% GUTTER GRADE ALONG CURB FACE. ENGINEER TO APPROVE FINAL CURBING CUT SHEETS PRIOR TO INSTALLATION. 4. SUBBASE MATERIAL FOR SIDEWALKS, CURB, OR ASPHALT SHALL BE FREE OF ORGANICS AND OTHER UNSUITABLE MATERIALS. SHOULD SUBBASE BE DEEMED UNSUITABLE, SUBBASE IS TO BE REMOVED AND FILLED WITH APPROVED FILL MATERIAL COMPACTED TO 95% OPTIMUM DENSITY (AS DETERMINED BY MODIFIED PROCTOR METHOD).
- 5. REFER TO SITE PLAN FOR ADDITIONAL NOTES. 6. IN CASE OF DISCREPANCIES BETWEEN PLANS, THE SITE PLAN WILL SUPERCEDE IN ALL CASES. CONTRACTOR MUST NOTIFY ENGINEER OF RECORD OF ANY CONFLICT IMMEDIATELY.
- 7. MAXIMUM CROSS SLOPE OF 2% ON ALL SIDEWALKS.
- 8. CONTRACTOR TO ENSURE A MAXIMUM OF 2% SLOPE IN ALL DIRECTIONS IN ADA PARKING SPACES AND ADA ACCESS AISLES. CONTRACTOR TO ENSURE A MAXIMUM OF 5% RUNNING SLOPE AND 2% CROSS SLOPE ALONG ALL OTHER PORTIONS OF ACCESSIBLE ROUTE, WITH THE EXCEPTION OF RAMPS AND CURB RAMPS. CONTRACTOR SHALL CLARIFY ANY QUESTIONS CONCERNING CONSTRUCTION IN ADA AREAS WITH THE ENGINEER PRIOR TO THE START OF CONSTRUCTION.
- 9. THE OWNER SHALL RETAIN DYNAMIC EARTH, LLC (908–879–7095) OR ALTERNATE QUALIFIED GEOTECHNICAL ENGINEER TO TEST SOIL PERMEABILITY AND PROVIDE CONSTRUCTION PHASE INSPECTIONS OF THE BASIN BOTTOM SOILS AND ANY FILL MATERIALS WITHIN ANY PROPOSED INFILTRATION OR RETENTION BASIN TO COMPARE RESULTS TO DESIGN CRITERIA.
- 10. CONTRACTOR IS TO REMOVE EXISTING UNSUITABLE OR OVERLY COMPACT SOIL OR ROCK AS NEEDED TO ACHIEVE REQUIRED PERMEABILITY AS DIRECTED BY THE OWNERS GEOTECHNICAL ENGINEER, AND NEW FILL, IF NEEDED, SHALL HAVE AN IN PLACE PERMEABILITY GREATER THAN OR EQUAL TO THE DESIGN CRITERIA. 11. CONTRACTOR IS RESPONSIBLE FOR CONTACTING THE OWNER'S GEOTECHNICAL ENGINEER PRIOR TO ONSET OF CONSTRUCTION TO SUBMIT AND CONFIRM THE CONTRACTOR'S PROPOSED MEANS AND MATERIALS AND TO SCHEDULE INSPECTIONS FOR BOTTOM OF BASIN, REMOVAL OF UNSUITABLE SOIL, FILL PLACEMENT, AND FINAL BASIN PERMEABILITY TESTING.

12. THE CONTRACTOR IS RESPONSIBLE FOR AS-BUILT PLANS AND GRADE CONTROL UNLESS DEFINED OTHERWISE ELSEWHERE IN THE CONTRACT DOCUMENTS.

ADA NOTES

ALL SLOPES INDICATED ARE ACTUAL. CONTRACTOR TO REFER TO LATEST ADA GUIDELINES AND NJ BARRIER FREE SUBCODE (NJAC 5:23-7) FOR SLOPE LIMITS. AT THE TIME OF PLAN DESIGN, THE SLOPE LIMITS ARE AS FOLLOWS:

- SIDEWALKS/ ACCESSIBLE ROUTES - RUNNING SLOPE: 1:20 (5%) MAX. (4.5% MAX. FOR NEW CONSTRUCTION)
 - CROSS SLOPE: 1:48 (2.08%) MAX., 1.0% MIN. (1.5% MAX. FOR NEW CONSTRUCTION) - INTERSECTION SLOPE: 1:48 (2.08%) MAX. IN ALL DIRECTIONS (1.5% MAX. FOR NEW CONSTRUCTION)
 - CHANGE IN LEVELS: 1/4" MAX. HEIGHT OR 1/2" MAX. HEIGHT WITH BEVELED EDGE BEVELED EDGE SLOPE OF 1:2 (50%) MAX. - GAPS: ½" MAX. WIDTH ELONGATED OPENINGS SHALL BE PLACED SO LONG DIMENSION IS PERPENDICULAR TO PATH OF

CURB RAMP - SLOPE 1:12 (8.3%) MAX. (7.4% MAX. FOR NEW CONSTRUCTION) - SIDE FLARE SLOPE: 1:10 (10%) MAX. (WHERE PEDS CROSS RAMP)

- BOTTOM LANDING: 48" MIN. LENGTH; WIDTH TO MATCH CURB RAMP; 1:48 MAX. (2.08%) IN ALL DIRECTIONS (1.5% MAX. FOR NEW CONSTRUCTION)
- TOP LANDING: 36" MIN. LENGTH; WIDTH TO MATCH CURB RAMP; 1:48 MAX. (2.08%) CROSS SLOPE (1.5% MAX. FOR NEW CONSTRUCTION) AND 1:20 (5%) RUNNING SLOPE (4.5% MAX FOR NEW CONSTRUCTION)

ACCESSIBILITY PARKING STALLS – SPACE AND ACCESS AISLE SLOPE: 1:48 MAX. (2.08%) IN ALL DIRECTIONS (1.5% MAX. FOR NEW CONSTRUCTION)

- CROSSWALKS RUNNING SLOPE: 1:20 (5%) MAX. (4.5% MAX. FOR NEW CONSTRUCTION) - CROSS SLOPE: 1:48 (2.08%) MAX. (1.5% MAX. FOR NEW CONSTRUCTION)
- CHANGE IN LEVELS: 4" max. Height or 1/2" max. Height with beveled edge. Beveled edge slope of 1:2 (50%) max. - GAPS: 1/2" MAX. WIDTH ELONGATED OPENINGS SHALL BE PLACED SO LONG DIMENSION IS PERPENDICULAR TO PATH OF
- TRAVFI <u>RAMPS</u>
- SLOPE: 1:12 (8.3%) MAX. (7.4% MAX. FOR NEW CONSTRUCTION) EXISTING RAMPS; SLOPE: 1:10 (10%) MAX. FOR RISE OF 6"; 1:8 (12.5%) MAX. FOR MAX. RISE OF 3"
- MAX. RISE: 30" – MIN. CLEAR WIDTH: 36"
- MIN. LANDING CLEAR LENGTH: 60" - MAX. CROSS SLOPE: 1:48 (2.08%) (1.5% MAX. FOR NEW CONSTRUCTION)





INLET AREA MAP EXHIBIT PROJECT: NAKT REAL ESTATE HOLDINGS, LLC 4087-99-001 09/16/2022 PROPOSED MEDICAL BUILDING SCALE: (H) 1"=20 DRAWN BY: BLOCK 473, LOT 1 DJB 49 SOUTH AVENUE WEST (CR 610) & LINCOLN AVENUE WEST DESIGNED BY: TOWNSHIP OF CRANFORD, UNION COUNTY, NEW JERSEY MPD SHEET No: CHECKED BY: JEH CHECKED BY: JAMES E. HENRY JOHN A. PALUS PROTECT YOURSELF All states require notification of excavators, designers, or any person preparing to disturb the earth's surface anywhere in any state OF í PROFESSIONAL ENGINEER Know what's Delow Call before you da. PROFESSIONAL ENGINEER NEW JERSEY LICENSE No. 49266 NEW JERSEY LICENSE No. 41975 FOR STATE SPECIFIC DIRECT PHONE NUMBERS VISIT: Rev. # O